



Redefining possible.

BUILDING AERODYNAMICS – OPTIMIZATION OF WIND- INDUCED STRUCTURAL RESPONSES

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Rowan Williams Davies & Irwin Inc. (RWDI)

RWDI – Company background

Established in 1972

450+ employees

Global presence

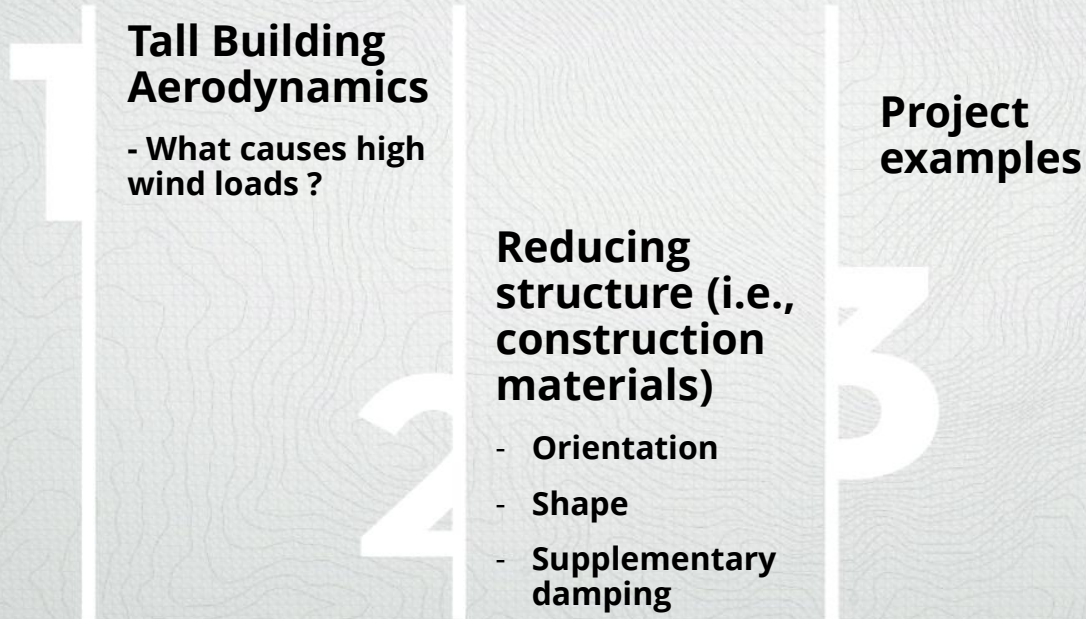
In Indonesia since 90's

Three Practice Areas:

- Climate Engineering
- Building Performance
- Environmental Engineering



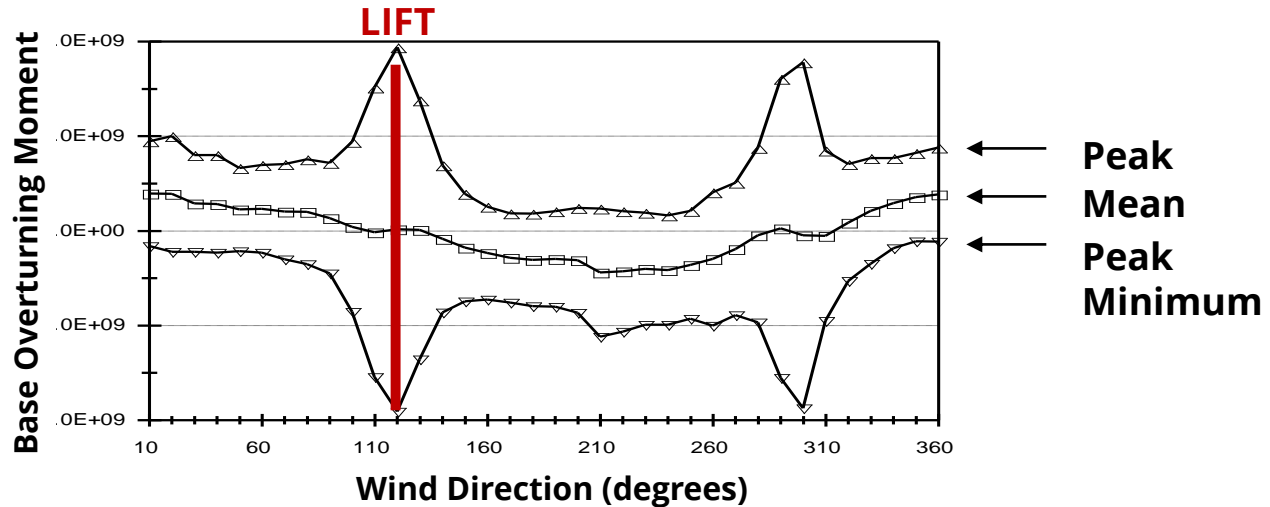
Talk Overview



What causes high
wind loads ?

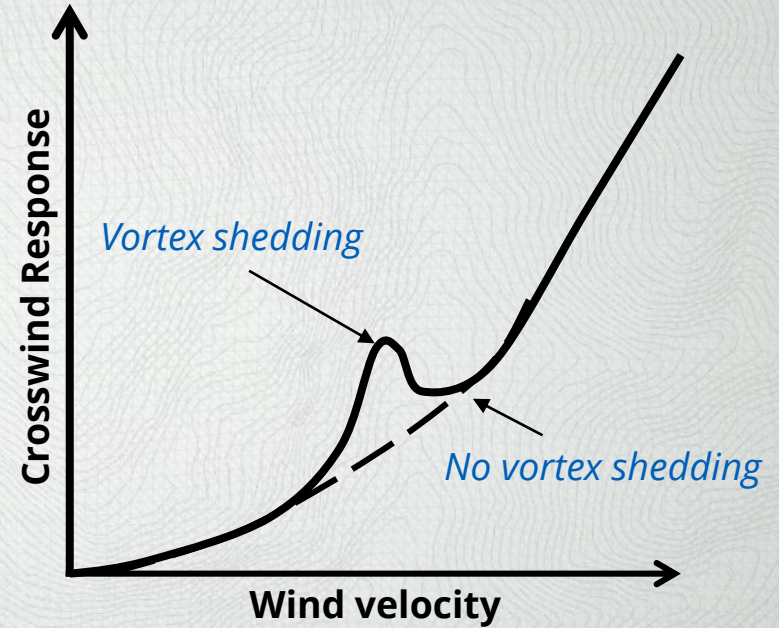
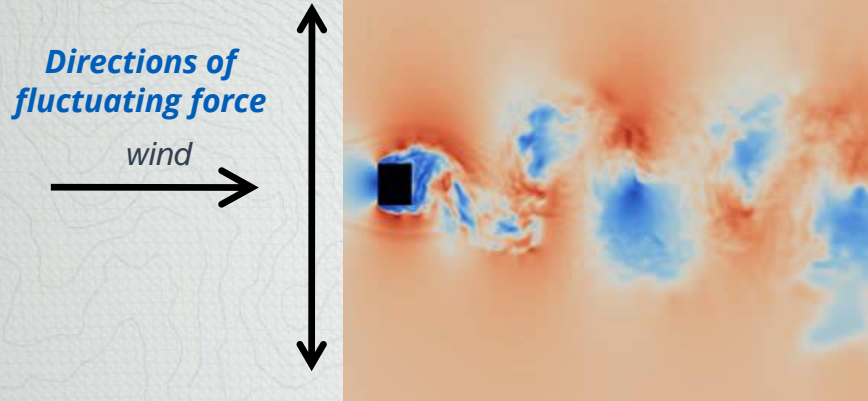
Response of a Typical Tall Building – Lift (Across Wind)

Across-wind response where mean loads are negligible



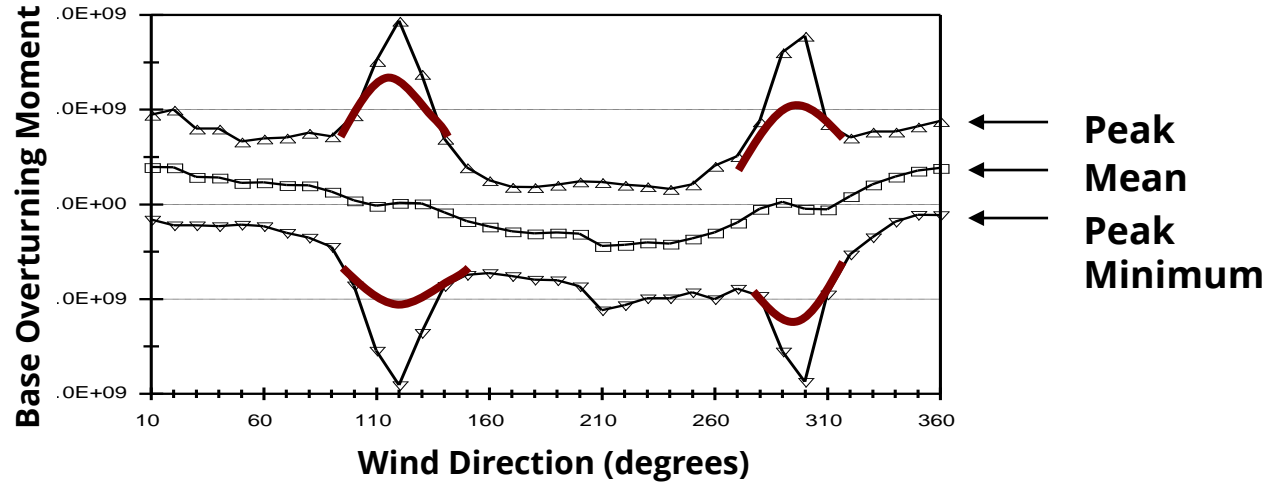
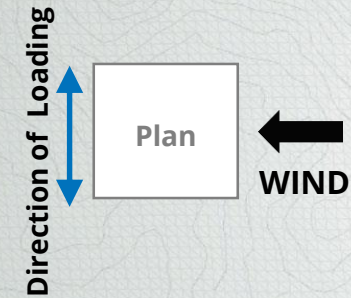
- High across-wind responses impact strength and serviceability design.
- **Requires additional structure to counter the problem.**

Across-Wind Loading (Vortex Shedding)



- Sometimes vortex shedding affects serviceability design only
- Requires additional structure to satisfy serviceability demands

Reduction of structure through wind tunnel testing

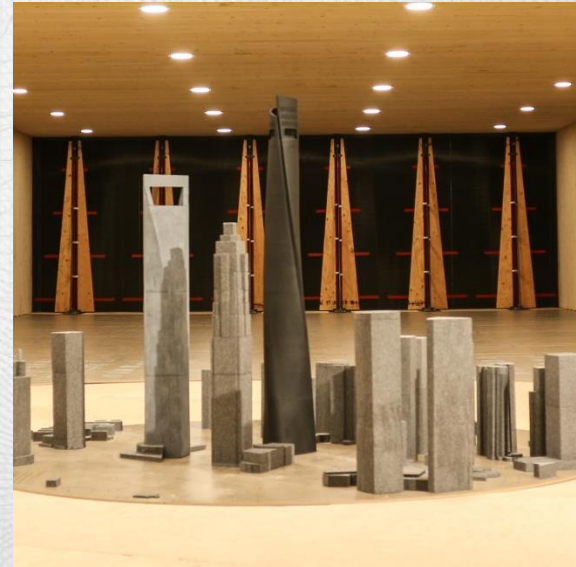


- Reduction of wind loads often translates into **reduced construction materials**
- Realizing material savings translates into other benefits such as emission reductions for transportation of materials from source to site

Wind Tunnel Testing

Wind tunnel testing accounts for project specific

- **Wind climate**
- **Aerodynamic shape**
- **Immediate Surroundings and Upwind Terrain Conditions**
- **Detailed structural properties (mass and stiffness designed by structural engineer)**
- **Damping (inherent and/or supplementary)**

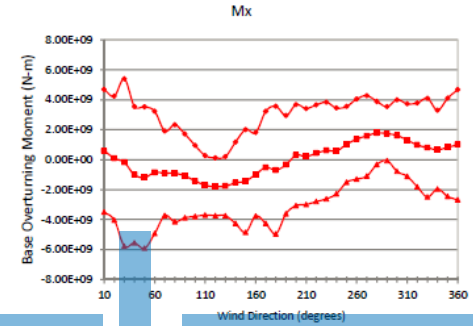
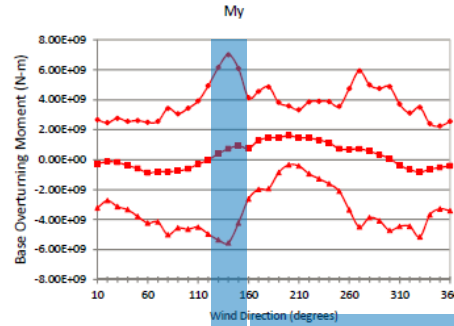


Influence of the Wind Climate

Aerodynamic Response

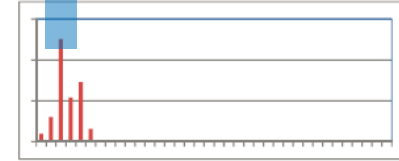
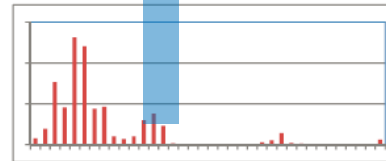
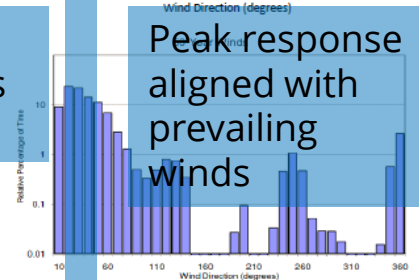
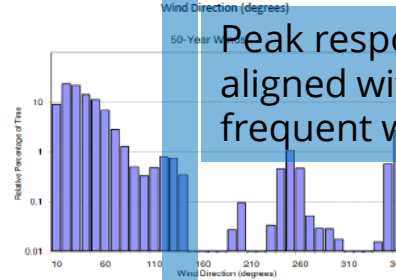
Wind Climate Model

**Combination of Response
and Directionality**



Peak response
aligned with less
frequent winds

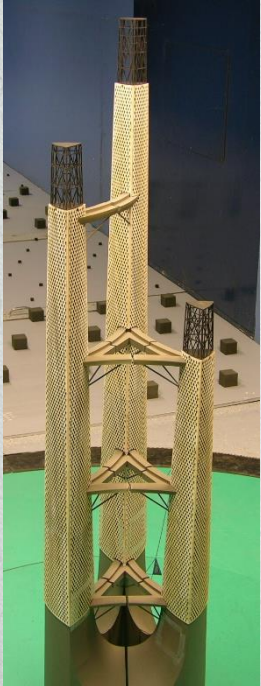
Peak response
aligned with
prevailing
winds



Requires less structure

Requires more structure

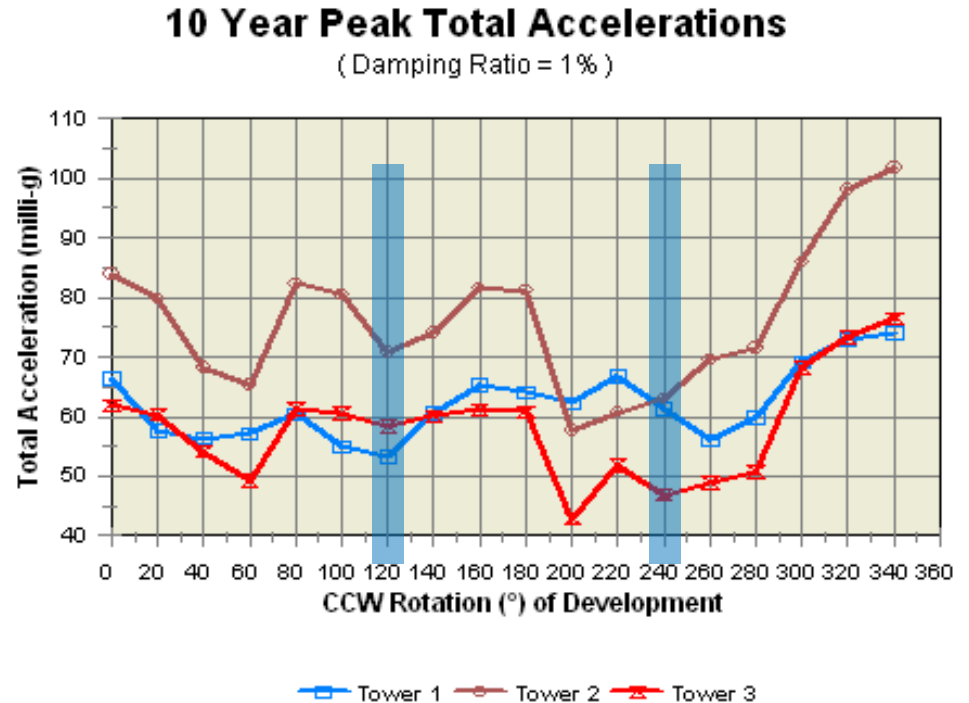
Influence of the Wind Climate – Three Tower Complex



Three linked supertall towers of varying heights with strong interactions between towers – structural and aerodynamic

Data analyzed for rotation of the wind climate at 20° increments around the compass

Apparent that certain orientations are more favorable

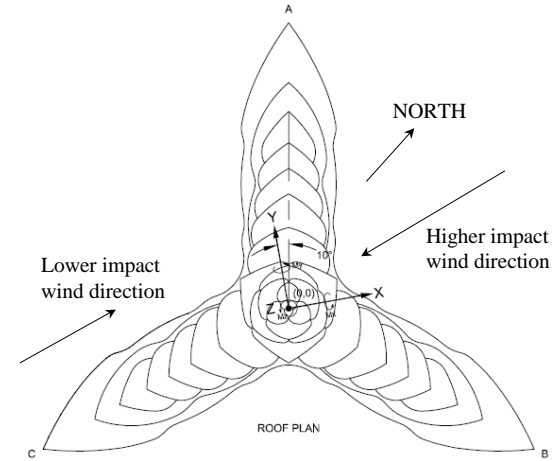
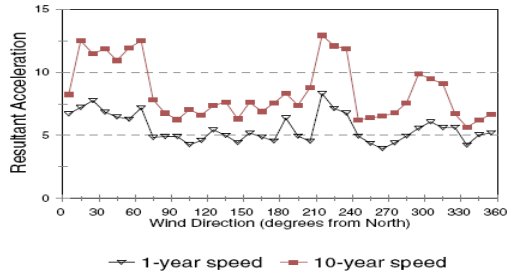
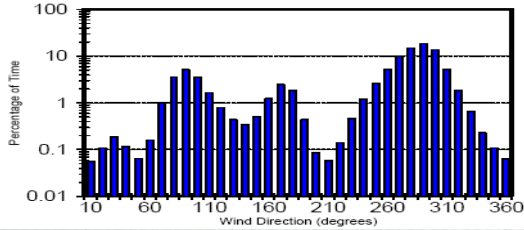




Optimizing building form

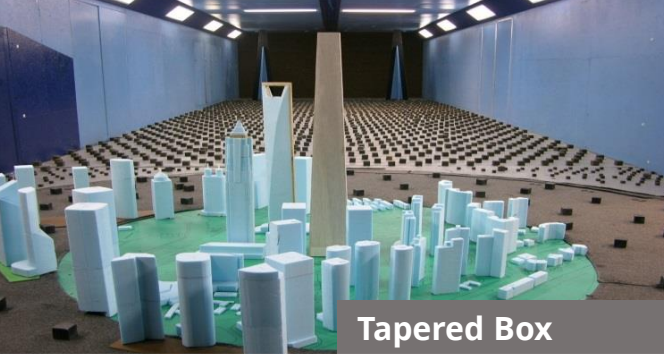
Burj Khalifa: Changing Cross Section, Orientation & Taper.

Winds Exceeding 31.0 m/s
5 Year Return Period



"We virtually designed [the tower] in a wind tunnel"

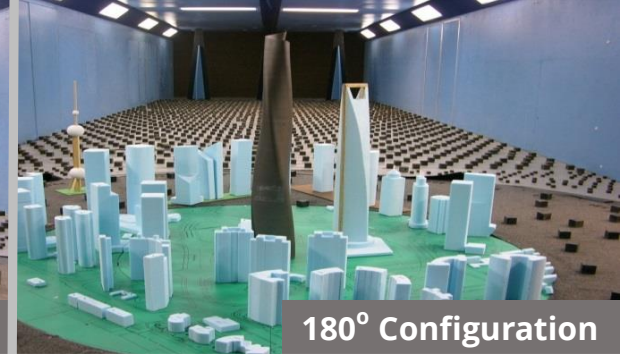
Bill Baker of SOM
Discussing the Burj Khalifa Project



Tapered Box



120° Configuration



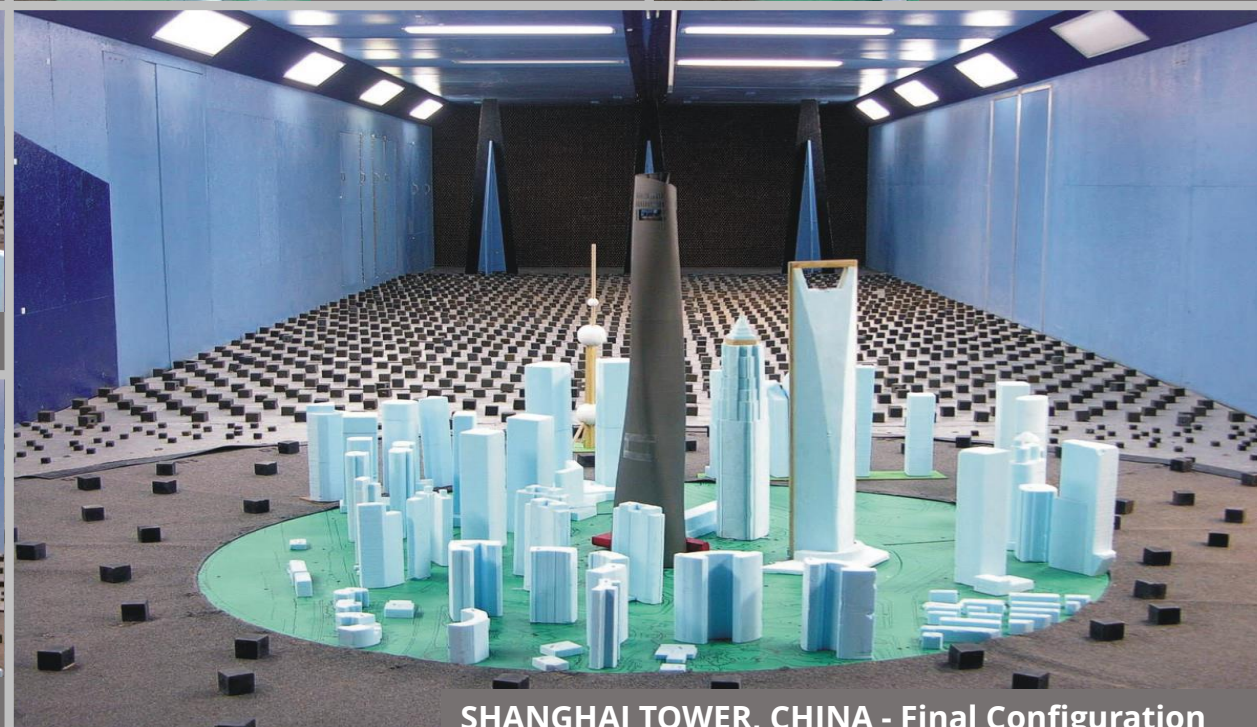
180° Configuration



100° Configuration



110° Configuration



SHANGHAI TOWER, CHINA - Final Configuration

Comparison of Base Overturning Moments

Assume the same structural properties for all configurations

(Vr=52m/s, 100-yr wind, damping=2.0%)

**32% reduction
in construction
materials !!!**

Benefits of Optimization due to Twist & Building Orientation

Reference →

Configuration	Test Date	My (N-m)	Ratio	Mx (N-m)	Ratio	Ref. Resultant	Ratio
Base (Tapered Box)	08/22/2008	5.45E+10	100%	4.98E+10	100%	6.22E+10	100%
100° (107°)	07/28/2008	4.53E+10	83%	4.19E+10	84%	5.18E+10	83%
110° (118°)	08/22/2008	3.97E+10	73%	4.31E+10	87%	4.92E+10	79%
180° (193°)	07/28/2008	3.39E+10	62%	3.65E+10	73%	4.18E+10	67%
120° (129°) - 0° Rot.	Estimated	3.43E+10	63%	4.29E+10	86%	4.75E+10	76%
110° (118°) - 30° Rot.	09/29/2008	3.92E+10	72%	3.60E+10	72%	4.48E+10	72%
120° - 40° Rot.	09/29/2008	3.57E+10	66%	3.53E+10	71%	4.15E+10	67%

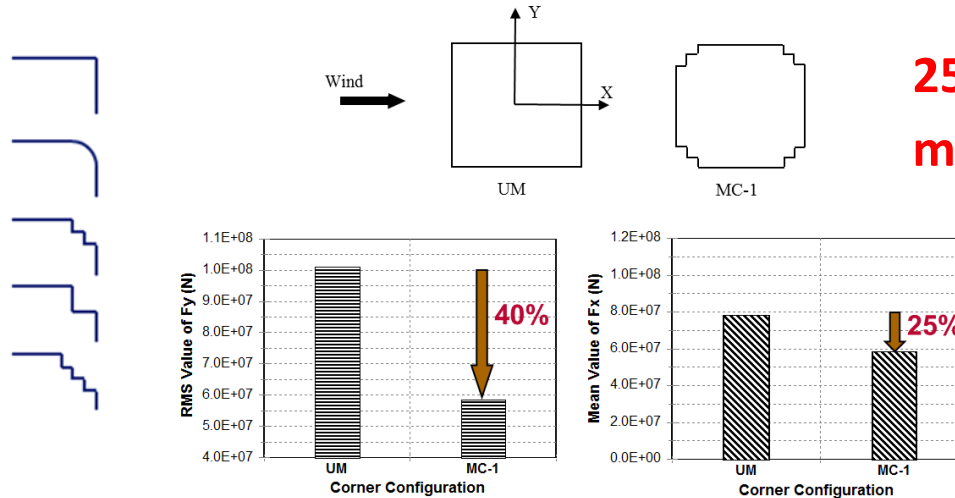
$$\text{Ref.Resultant} = \sqrt{(Max)^2 + (0.6 \times Min)^2}$$

Final Configuration

0° Rot. – Original 110° Shape Footprint Position
30° Rot. – Optimal Orientation of 110° Shape
40° Rot. – Optimal Orientation of 120° Shape



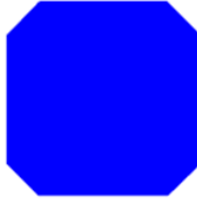
TAIPEI 101: Sensitivity to Corner Details



Sensitivity to Corner Details



**Slotted
Corners**



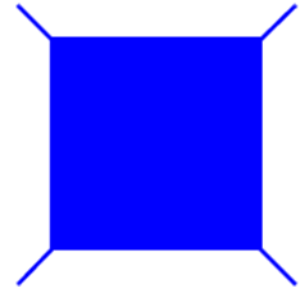
Chamfered Corners



"Softened" corners



Basic

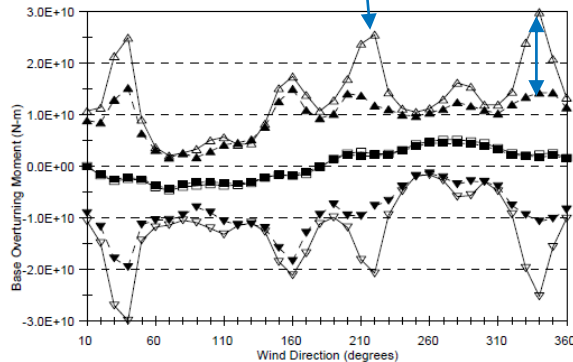


Fins

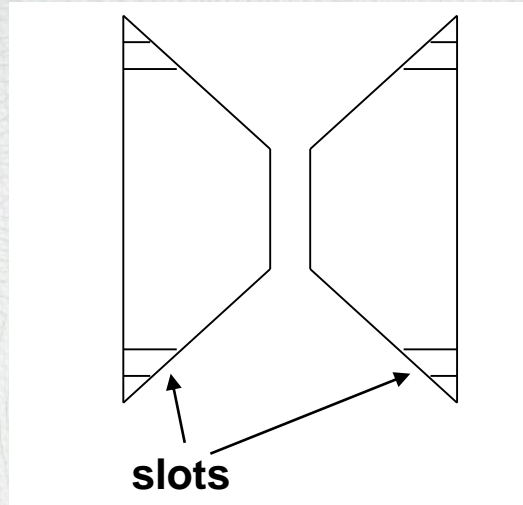
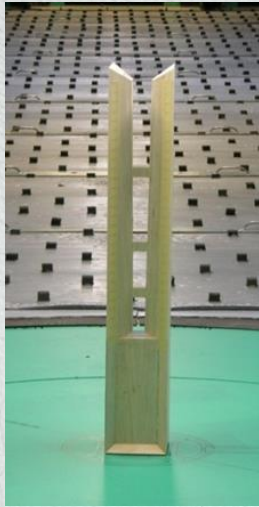
"Hardened" corners

Exposed structure at edges

Big increase
in response



151 Incheon Tower – Korea: Corner openings



- In order to mitigate the high vortex shedding responses a variety of aerodynamic modifications were explored which focused on changes to outer corners with the inclusion of slots in the building.
- Modified roof, made more porous
- **60% reduction in base moments**

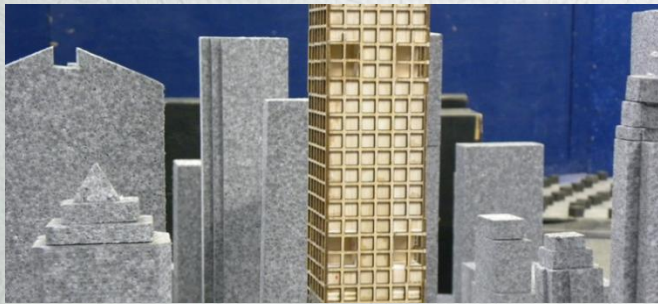
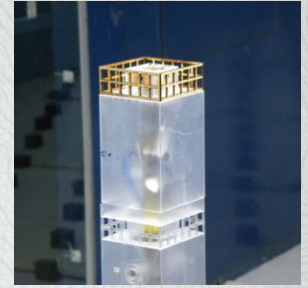
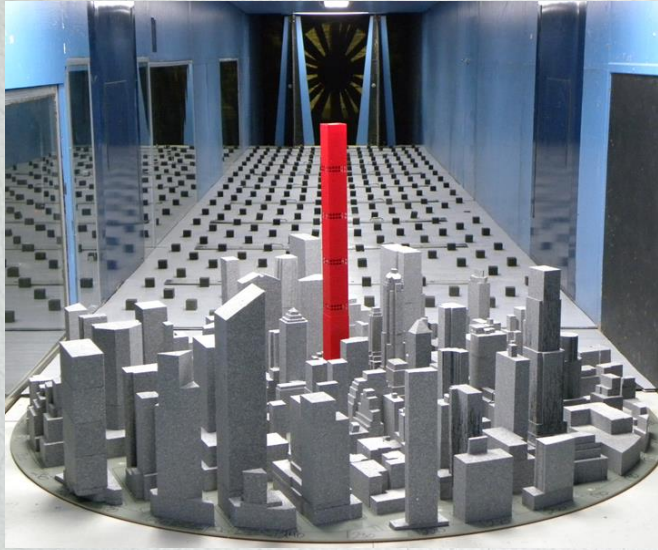
432 Park Avenue – NYC

96 Stories
Residential Tower
(15:1 Aspect Ratio)

Period >> 10 secs



Exploration of Openings



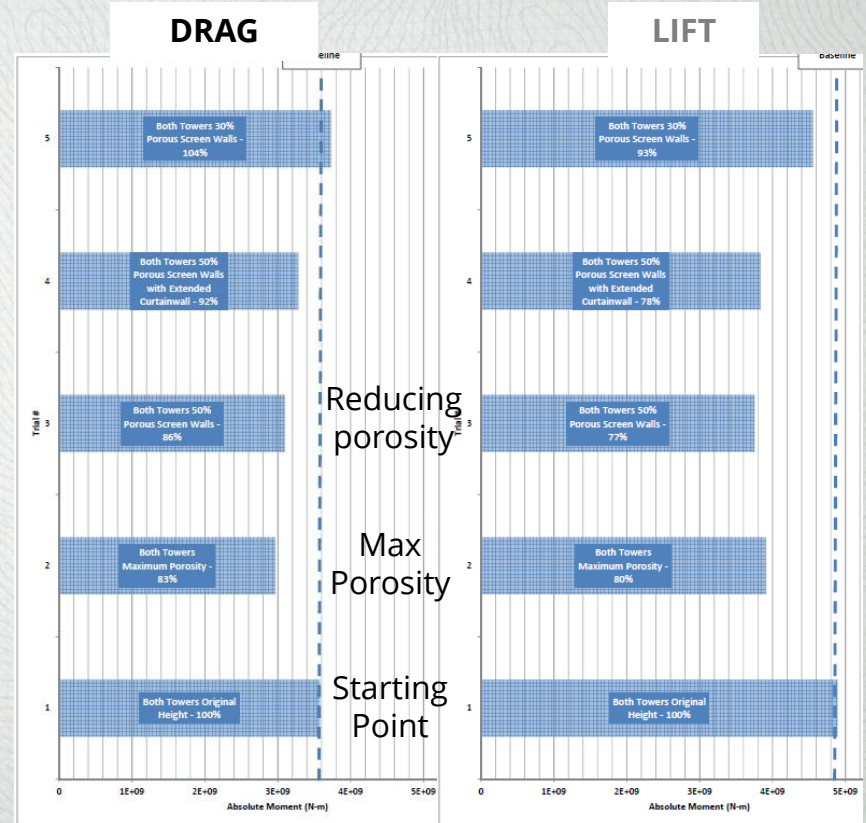


Twin Towers – Fine tuning for Height Increase

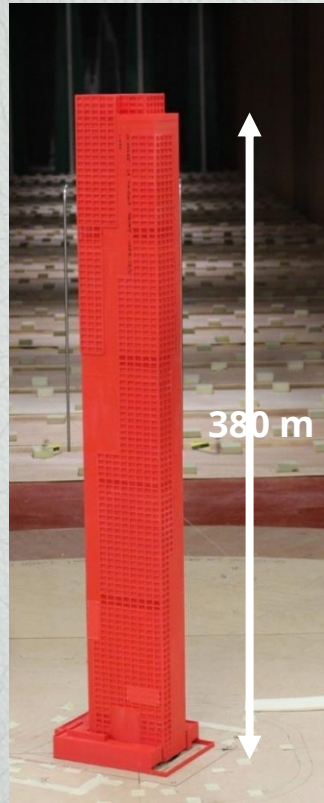
Design completed
based on wind
tunnel testing

Desired to increase
the height **without**
increasing base
loads

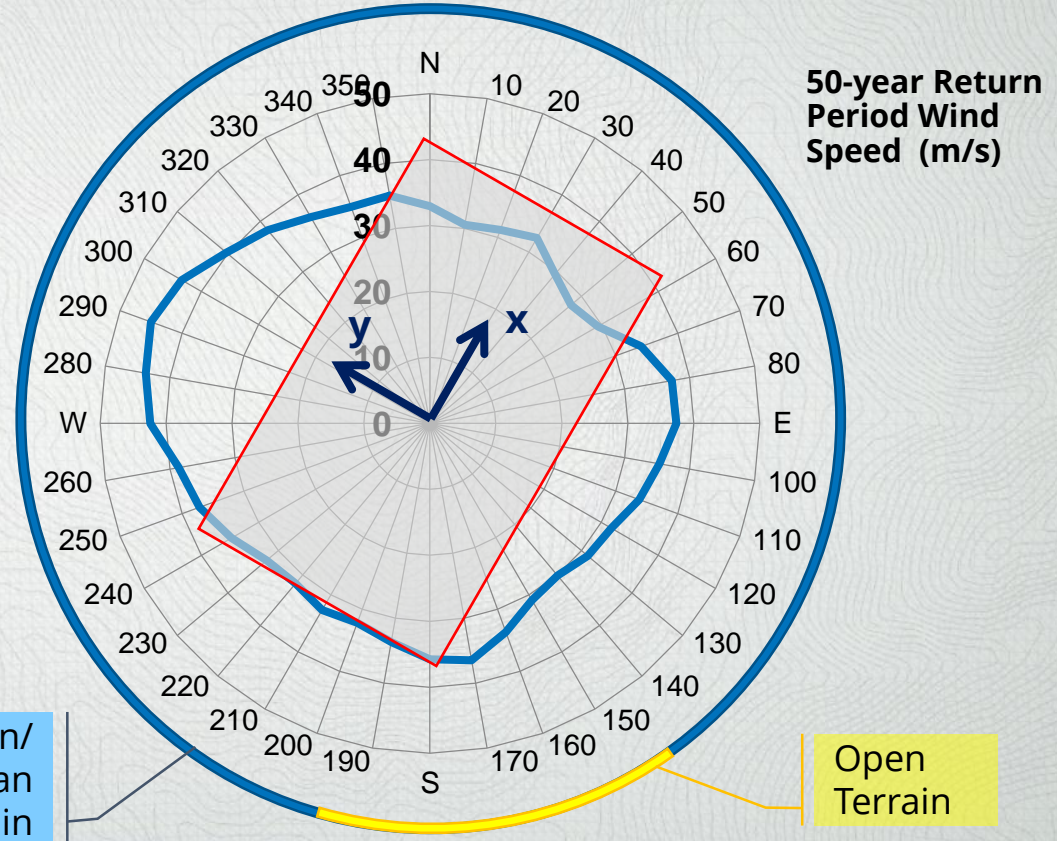
Porosity introduced
at the top



380 m Residential Tower

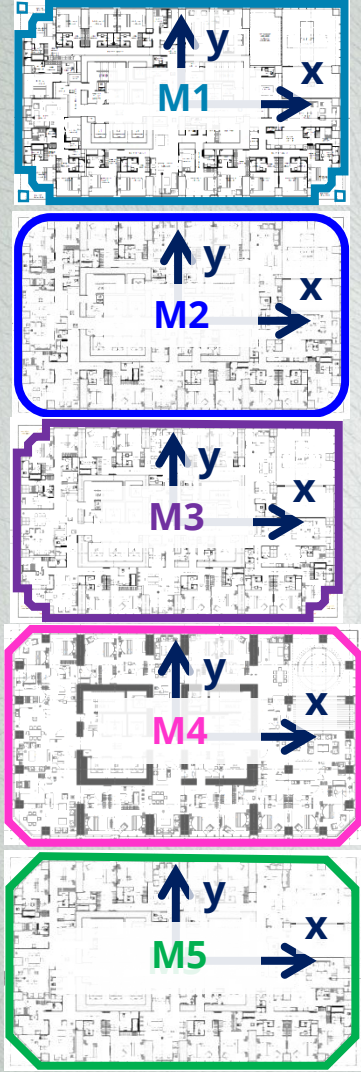
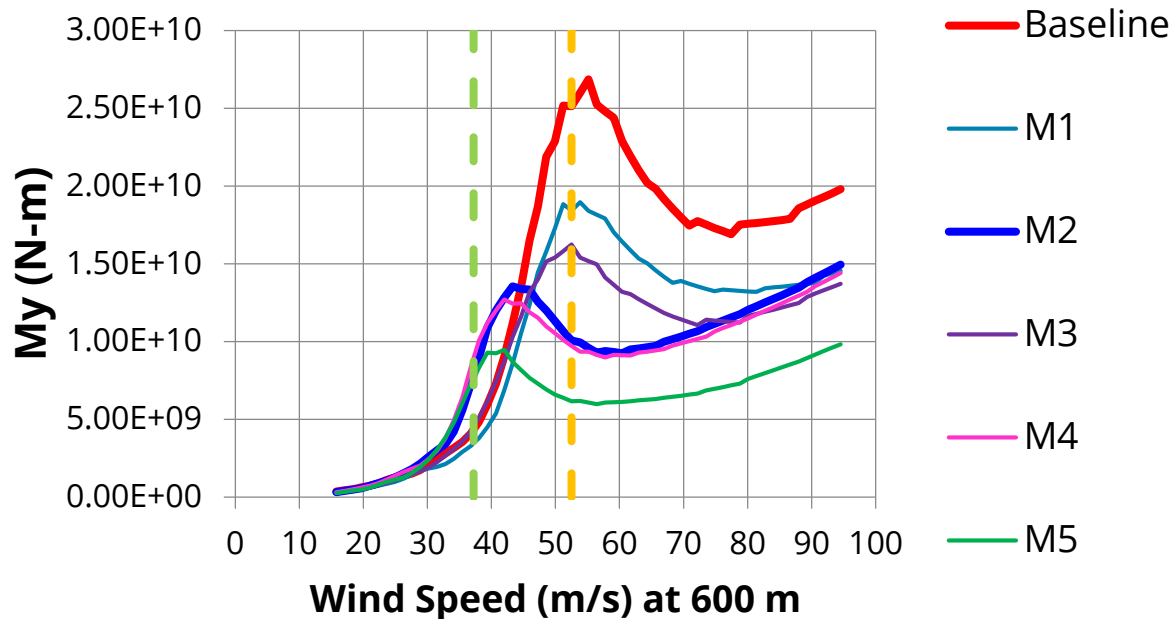


1:400 scale



My Base Moment vs Wind Speed, angle 300°

40% reduction in base moments





Influence of damping

Damping in Structures – How much?

Considerable scatter in available data

Tall buildings certainly don't seem predisposed to *high* levels of as-built inherent structural damping

Damping is often observed to be amplitude-dependent

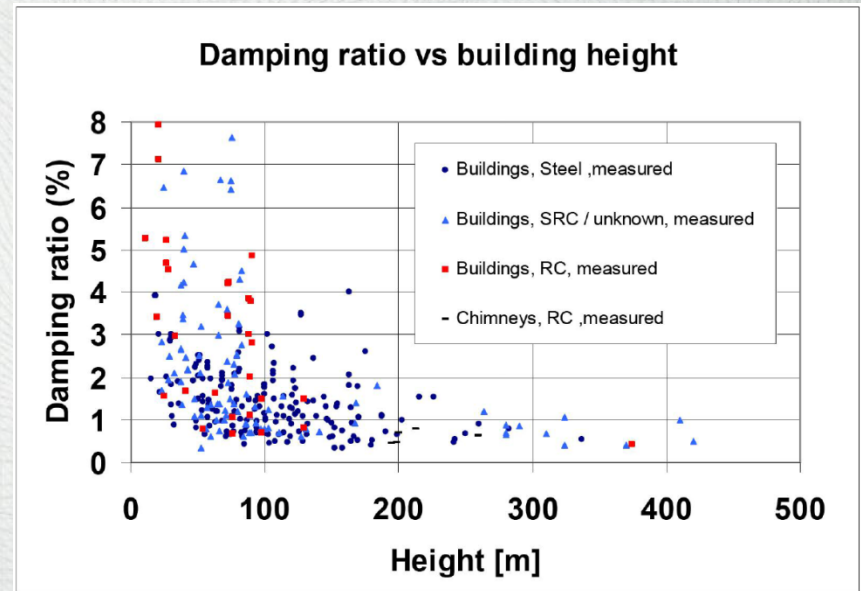
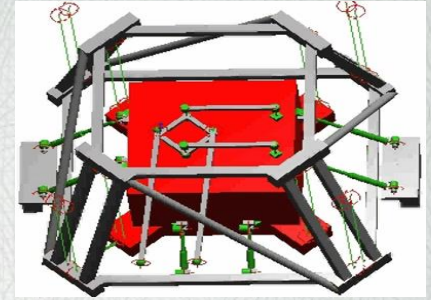


Image Credit: Smith & Willford, Arup, "Damping in tall buildings – uncertainties and solutions", 17th Congress of IABSE, 2008

Types of Supplemental Damping Systems

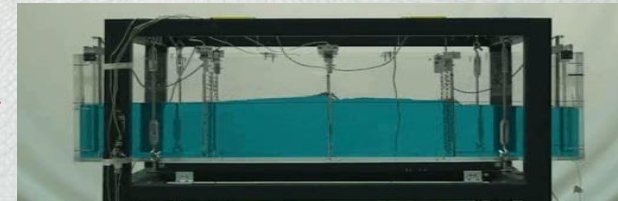
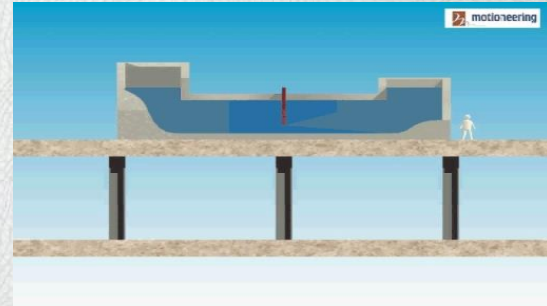
Solid Mass Type:

- Tuned Mass Damper (TMD)
- Various configurations possible



Water/Liquid Type:

- Tuned Liquid Column Damper (TLCD)
- Tuned Sloshing Damper (TSD)



Benefits of Supplementary Damping for Buildings

- Achieve “High-Quality Building” standard in terms of comfort
- Save quantity of structure needed cost savings
- Increased revenue through increased floor space
- Sustainable Design / Green Benefits

Case Study Example

75-storey concrete, residential tower in an urban location

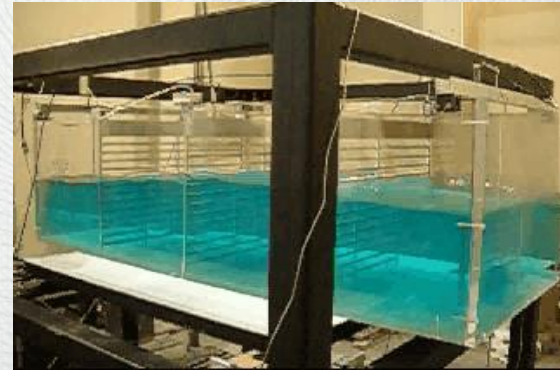
Early design involvement with RWDI

Tuned Sloshing Damper used to *optimize structure*

At least 3% damping WILL be achieved

Case A: explore adding floors without thickening shear walls

Case B: explore reductions in structure



Tuned Sloshing Damper
– integrated into design **EARLY**



Results – Case A

5 more floors, no increase in shear walls,
additional materials

Concrete:	2,100 m ³
Reinforcement:	250,500 kg
Formwork:	10,600 m ²
INCREASED COST:	\$1.6 Million

Gain of Usable Floor Space:	60,000 ft ²
<u>INCREASED REVENUE:</u>	<u>\$36 Million</u>



Results – Case B

Reduction in Structural Materials

Concrete:	1,400 m ³
Reinforcement:	88,000 kg
PT Strand:	9,300 kg
COST SAVINGS:	\$450,000

Gain of Usable Floor Space:	2,800 ft ²
COST SAVINGS: 875 tons of CO₂	
<u>INCREASED REVENUE:</u>	<u>\$1.7 Million</u>

Equivalent to over 68,000 cars
off the road for one day





THANK YOU 😊

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